

- Notes
1. Do not scale this drawing.
 2. This drawing is to be read in conjunction with all other project drawings and specifications.
 3. All dimensions are in millimetres unless stated otherwise.
 4. Should there be any conflict between the details indicated on this drawing and those indicated on other drawings the Project Engineer shall be informed prior to construction.
 5. Until technical approval has been obtained from the relevant authority, it should be understood that all drawings issued are preliminary and not for construction. Should the contractor commence site work prior to such approval being given, it is entirely at their own risk.
 6. Drawing to be read in conjunction with 1550-002.
 7. All 1000 proposed drainage pipes shown are to be laid at a minimum gradient of 1:80.
 8. All 1500 proposed drainage pipes shown are to be laid at a minimum gradient of 1:150.
 9. All existing land drains encountered on site during construction are to be re-connected.
 10. Temporary protection to be provided to drainage work during construction as necessary.
 11. Topographical survey and architectural layout based on third party information.
 12. Anticipated foul flow rates calculated using discharge unit method to BS EN 12056-2.
 13. Drawing to be read in conjunction with Causeway Flow design pack.
 14. Pipes to be structured walled to BS EN 13476, Polypropylene to BE EN 1852 or PVC-U to BS EN 1401.
 15. Both clay and concrete pipes shall be strength class 120 (100/150mm min crushing strength 28kN/m). Thermoplastic pipes shall have a minimum ring stiffness of SN4.
 16. Pipes which run adjacent to buildings shall be installed in strict accordance with Building Regulations Part H, clauses 2.23 to 2.25.
 17. Class Z concrete bed and surround to all foul and surface water pipes with less than 900mm cover depth. Type 5 granular bed and surround to all foul and surface water pipes with greater than 900mm cover depth.
 18. All manholes and inspection chambers subject to vehicular trafficking to have D400 load-rated covers and frames to BS EN 124. All manholes and inspection chambers not subject vehicular trafficking to have B125 load-rated covers and frames to BS EN 124.
 19. Concrete to be GEN1 unless specified otherwise.
 20. The first flexible joint in pipes adjoining a manhole shall be a maximum length of 600mm from the inside face of the manhole, connecting to a rocker pipe. The length of the rocker pipe shall be 600mm.
 21. All foul and surface water pipes to be constructed to Building Regulations Part H.
 22. Geo-cellular units to be PolyStorm PSM1A or similar approved.
 23. Geo-cellular units to be installed in accordance with manufacturer's instructions and specification.
 24. Manufacturer to provide structural calculations to relevant industry guidance which confirm the product is suitable for use at the proposed depth and expected loading conditions.
 25. Hydro-Brake devices or similar approved to achieve flow restrictions as shown.
 26. Connectivity of all existing site drainage to be confirmed prior to construction.

CDM Requirements

Risk - Deep excavation

Control method - Contractor to design trench support to depths shown on drawings and in the manhole schedule, appropriate to the ground conditions.

Risk - Water ingress into excavations, including ground water

Control method - Contractor to specify method of dealing with ingress of water into excavations, in particular if ground water is experienced. Contractor to undertake trench inspections prior to entry into any excavation, and again if left overnight or if conditions change.

Key



Proposed Exceedance Route

+100.000

Proposed Spot Level



NC29 Ltd

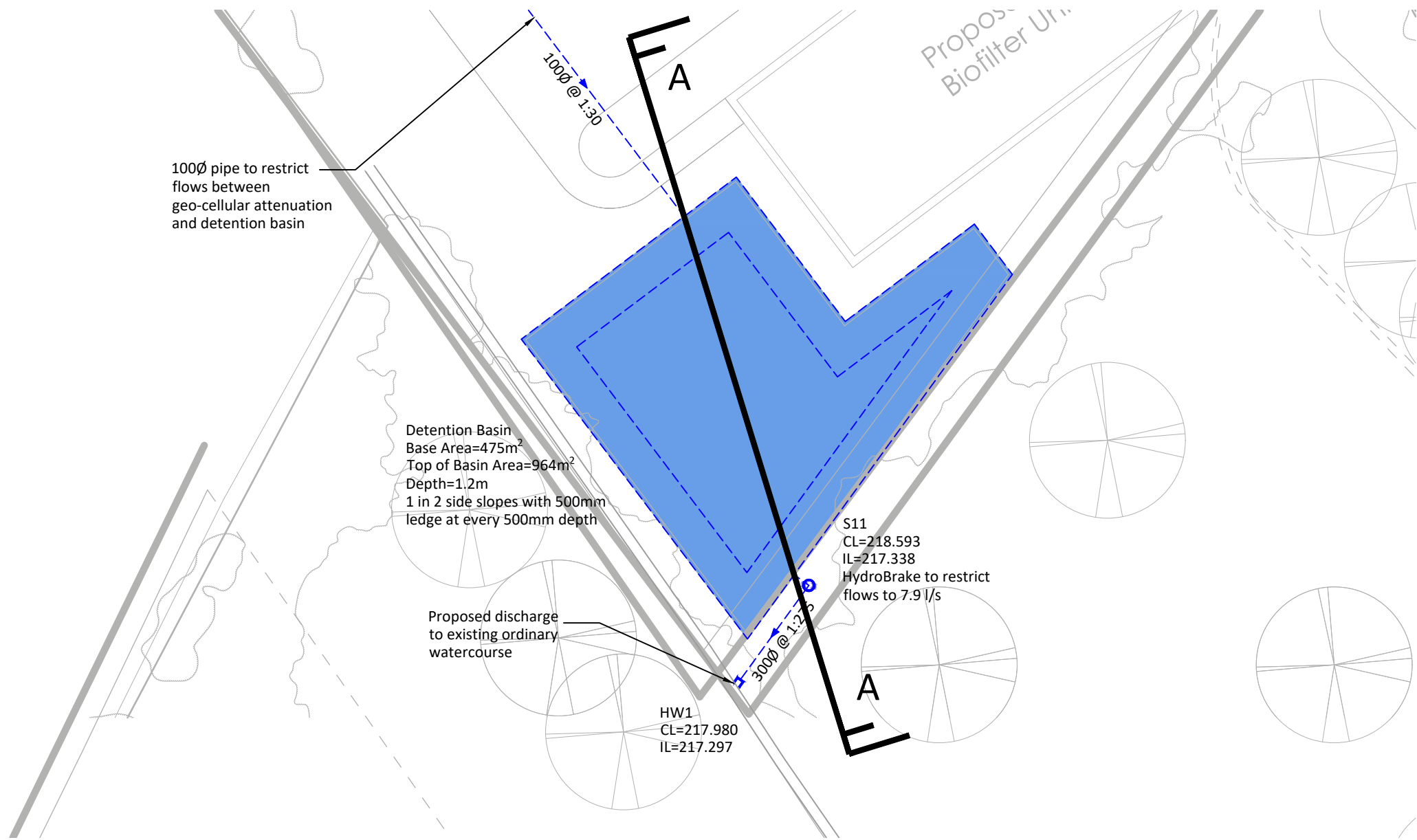
Newstead House, Pelham Road
Nottingham, NG5 1AP
Tel : 07753 168226 Web: www.nc29.co.uk
Email : info@nc29.co.uk

Project **Proposed Food Processing Unit
Bones Lane, Cheddleton**

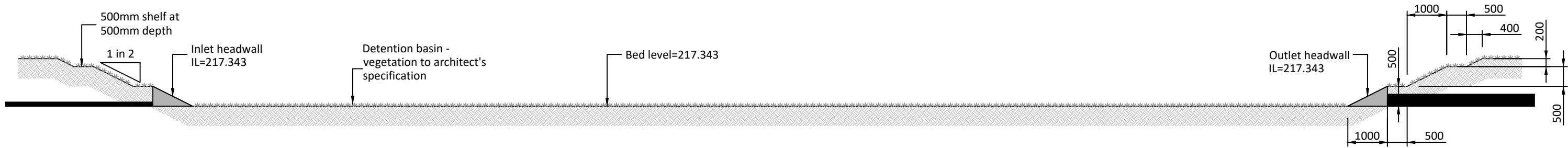
Client **Advanced Proteins Ltd**

Title **Exceedance Routes Plan**

Drawn	CC	Checked	CC
Date	July 2026	Scale	1:500
Number	2191/005 A	Status	Preliminary



Proposed Detention Basin Layout
Scale 1:500



Section A-A
Section Through Proposed Detention Basin Layout
Scale 1:100

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- Pipes which run adjacent to buildings shall be installed in strict accordance with Building Regulations Part H, clauses 2.23 to 2.25.
- Class Z concrete bed and surround to all foul and surface water pipes with less than 900mm cover depth. Type S granular bed and surround to all foul and surface water pipes with greater than 900mm cover depth.
- All manholes and inspection chambers subject to vehicular trafficking to have D400 load-rated covers and frames to BS EN 124. All manholes and inspection chambers not subject to vehicular trafficking to have B125 load-rated covers and frames to BS EN 124. Concrete to be GEN1 unless specified otherwise.
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- Geo-cellular units to be PolyStorm PSM1A or similar approved.
- Geo-cellular units to be installed in accordance with manufacturer's instructions and specification.
- Manufacturer to provide structural calculations to relevant industry guidance which confirm the product is suitable for use at the proposed depth and expected loading conditions.
- Hydro-Brake devices or similar approved to achieve flow restrictions as shown.
- Connectivity of all existing site drainage to be confirmed prior to construction.

CDM Requirements

Risk - Deep excavation

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Revision	Details	Date
NC29 Ltd Newstead House, Pelham Road Nottingham, NG5 1AP Tel : 07753 168226 Web: www.nc29.co.uk Email : info@nc29.co.uk		
Project	Proposed Food Processing Unit Bones Lane, Cheddleton	
Client	Advanced Proteins Ltd	
Title	Proposed Detention Basin Section	
Drawn	CC	Checked CC
Date	Aug 2026	Scale Varies
Number	2191/006	Status Preliminary

Appendix D – Hydraulic Calculations

Design Settings

Rainfall Methodology	FEH-22	Minimum Velocity (m/s)	1.00
Return Period (years)	2	Connection Type	Level Soffits
Additional Flow (%)	0	Minimum Backdrop Height (m)	2.000
CV	1.000	Preferred Cover Depth (m)	0.950
Time of Entry (mins)	4.00	Include Intermediate Ground	✓
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	x
Maximum Rainfall (mm/hr)	50.0		

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Depth (m)	Invert Level (m)
S1	0.140	4.00	228.450	1200	1.250	227.200
S2			227.750	1350	1.325	226.425
S3	0.082	4.00	227.797	1350	1.425	226.372
S4			227.380	1350	1.325	226.055
S5	0.223	4.00	227.041	1350	1.641	225.400
S6	0.223	4.00	224.500	1350	1.400	223.100
S7	0.261	4.00	222.671	1350	1.573	221.098
S8		4.00	225.665	1200	1.250	224.415
S9			222.000	1200	1.250	220.750
S10	0.295	4.00	221.033	1350	2.253	218.780
S11			218.593	1200	1.255	217.338
S12			218.552	1200		
S13			218.552	1200		
ATTENUATION			222.000		3.217	218.783
POND			218.593		1.250	217.343
HW1			218.552	1200	1.255	217.297

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1.000	S1	S2	26.278	0.600	227.200	226.500	0.700	37.5	300	4.17	50.0
1.001	S2	S3	17.066	0.600	226.425	226.372	0.053	322.0	375	4.45	50.0
1.002	S3	S4	19.789	0.600	226.372	226.055	0.317	62.4	375	4.60	50.0
1.003	S4	S5	39.289	0.600	226.055	225.400	0.655	60.0	375	4.88	50.0
1.004	S5	S6	66.094	0.600	225.400	223.175	2.225	29.7	375	5.21	50.0
1.005	S6	S7	64.209	0.600	223.100	221.098	2.002	32.1	450	5.50	50.0
1.006	S7	ATTENUATION	15.417	0.600	221.098	218.783	2.315	6.7	450	5.54	50.0
1.007	ATTENUATION	S10	1.000	0.600	218.783	218.780	0.003	333.3	450	5.55	50.0

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
1.000	2.574	181.9	25.3	0.950	0.950	0.140	0.0	75	1.831
1.001	1.004	110.9	25.3	0.950	1.050	0.140	0.0	121	0.818
1.002	2.296	253.6	40.1	1.050	0.950	0.222	0.0	100	1.692
1.003	2.343	258.8	40.1	0.950	1.266	0.222	0.0	99	1.720
1.004	3.335	368.3	80.4	1.266	0.950	0.445	0.0	118	2.689
1.005	3.599	572.4	120.7	0.950	1.123	0.668	0.0	139	2.877
1.006	7.914	1258.7	167.9	1.123	2.767	0.929	0.0	110	5.589
1.007	1.108	176.2	167.9	2.767	1.803	0.929	0.0	354	1.253

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
2.000	S8	S9	46.820	0.600	224.415	220.750	3.665	12.8	300	4.18	50.0
2.001	S9	S10	72.608	0.600	220.750	218.780	1.970	36.9	300	4.64	50.0
1.008	S10	POND	32.097	0.600	218.780	217.543	1.237	25.9	100	5.90	49.6
1.009	POND	S11	1.000	0.600	217.343	217.338	0.005	200.0	300	5.92	49.5
1.010	S11	HW1	11.200	0.600	217.338	217.297	0.041	275.0	300	6.12	48.8

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
2.000	4.421	312.5	0.0	0.950	0.950	0.000	0.0	0	0.000
2.001	2.598	183.6	0.0	0.950	1.953	0.000	0.0	0	0.000
1.008	1.521	11.9	219.2	2.153	0.950	1.224	0.0	100	1.562
1.009	1.108	78.3	219.0	0.950	0.955	1.224	0.0	300	1.122
1.010	0.943	66.7	215.7	0.955	0.955	1.224	0.0	300	0.955

Pipeline Schedule

Link	Length (m)	Slope (1:X)	Dia (mm)	Link Type	US CL (m)	US IL (m)	US Depth (m)	DS CL (m)	DS IL (m)	DS Depth (m)
1.000	26.278	37.5	300	Circular	228.450	227.200	0.950	227.750	226.500	0.950
1.001	17.066	322.0	375	Circular	227.750	226.425	0.950	227.797	226.372	1.050
1.002	19.789	62.4	375	Circular	227.797	226.372	1.050	227.380	226.055	0.950
1.003	39.289	60.0	375	Circular	227.380	226.055	0.950	227.041	225.400	1.266
1.004	66.094	29.7	375	Circular	227.041	225.400	1.266	224.500	223.175	0.950
1.005	64.209	32.1	450	Circular	224.500	223.100	0.950	222.671	221.098	1.123
1.006	15.417	6.7	450	Circular	222.671	221.098	1.123	222.000	218.783	2.767
1.007	1.000	333.3	450	Circular	222.000	218.783	2.767	221.033	218.780	1.803
2.000	46.820	12.8	300	Circular	225.665	224.415	0.950	222.000	220.750	0.950
2.001	72.608	36.9	300	Circular	222.000	220.750	0.950	221.033	218.780	1.953
1.008	32.097	25.9	100	Circular	221.033	218.780	2.153	218.593	217.543	0.950
1.009	1.000	200.0	300	Circular	218.593	217.343	0.950	218.593	217.338	0.955
1.010	11.200	275.0	300	Circular	218.593	217.338	0.955	218.552	217.297	0.955

Link	US Node	Dia (mm)	Node Type	MH Type	DS Node	Dia (mm)	Node Type	MH Type
1.000	S1	1200	Manhole	Adoptable	S2	1350	Manhole	Adoptable
1.001	S2	1350	Manhole	Adoptable	S3	1350	Manhole	Adoptable
1.002	S3	1350	Manhole	Adoptable	S4	1350	Manhole	Adoptable
1.003	S4	1350	Manhole	Adoptable	S5	1350	Manhole	Adoptable
1.004	S5	1350	Manhole	Adoptable	S6	1350	Manhole	Adoptable
1.005	S6	1350	Manhole	Adoptable	S7	1350	Manhole	Adoptable
1.006	S7	1350	Manhole	Adoptable	ATTENUATION		Junction	
1.007	ATTENUATION		Junction		S10	1350	Manhole	Adoptable
2.000	S8	1200	Manhole	Adoptable	S9	1200	Manhole	Adoptable
2.001	S9	1200	Manhole	Adoptable	S10	1350	Manhole	Adoptable
1.008	S10	1350	Manhole	Adoptable	POND		Junction	
1.009	POND		Junction		S11	1200	Manhole	Adoptable
1.010	S11	1200	Manhole	Adoptable	HW1	1200	Manhole	Adoptable

Manhole Schedule

Node	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)	
S1	228.450	1.250	1200					
				0	1.000	227.200	300	
S2	227.750	1.325	1350		1	1.000	226.500	300
				0	1.001	226.425	375	
S3	227.797	1.425	1350		1	1.001	226.372	375
				0	1.002	226.372	375	
S4	227.380	1.325	1350		1	1.002	226.055	375
				0	1.003	226.055	375	
S5	227.041	1.641	1350		1	1.003	225.400	375
				0	1.004	225.400	375	
S6	224.500	1.400	1350		1	1.004	223.175	375
				0	1.005	223.100	450	
S7	222.671	1.573	1350		1	1.005	221.098	450
				0	1.006	221.098	450	
S8	225.665	1.250	1200					
				0	2.000	224.415	300	
S9	222.000	1.250	1200		1	2.000	220.750	300
				0	2.001	220.750	300	
S10	221.033	2.253	1350		1	2.001	218.780	300
				2	1.007	218.780	450	
				0	1.008	218.780	100	
S11	218.593	1.255	1200		1	1.009	217.338	300
				0	1.010	217.338	300	
S12	218.552		1200					
S13	218.552		1200					

Manhole Schedule

Node	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)
ATTENUATION	222.000	3.217		1	1.006	218.783	450
				0	1.007	218.783	450
POND	218.593	1.250		1	1.008	217.543	100
				0	1.009	217.343	300
HW1	218.552	1.255	1200	1	1.010	217.297	300

Simulation Settings

Rainfall Methodology	FEH-22	Analysis Speed	Normal	Starting Level (m)	
Rainfall Events	Singular	Skip Steady State	x	Check Discharge Rate(s)	x
Summer CV	1.000	Drain Down Time (mins)	240	Check Discharge Volume	x
Winter CV	1.000	Additional Storage (m³/ha)	0.0		

Storm Durations

15 | 30 | 60 | 120 | 180 | 240 | 360 | 480 | 600 | 720 | 960 | 1440

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
2	0	0	0
30	0	0	0
100	40	0	0

Node S11 Online Hydro-Brake® Control

Flap Valve	x	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	x	Sump Available	✓
Invert Level (m)	217.338	Product Number	CTL-SHE-0127-7900-1255-7900
Design Depth (m)	1.255	Min Outlet Diameter (m)	0.150
Design Flow (l/s)	7.9	Min Node Diameter (mm)	1200

Node ATTENUATION Soakaway Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	218.783	Depth (m)	1.600
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)		Inf Depth (m)	
Safety Factor	2.0	Pit Width (m)	17.000	Number Required	1
Porosity	0.95	Pit Length (m)	29.000		

Node POND Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	217.343
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	475.0	0.0	1.200	964.0	0.0

Results for 2 year Critical Storm Duration. Lowest mass balance: 99.96%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	S1	10	227.281	0.081	27.1	0.0912	0.0000	OK
15 minute summer	S2	10	226.554	0.129	27.1	0.1839	0.0000	OK
15 minute summer	S3	10	226.482	0.110	42.9	0.1579	0.0000	OK
15 minute summer	S4	10	226.158	0.103	43.1	0.1469	0.0000	OK
15 minute summer	S5	10	225.524	0.124	86.1	0.1777	0.0000	OK
15 minute summer	S6	10	223.250	0.150	127.3	0.2151	0.0000	OK
15 minute summer	S7	10	221.222	0.124	176.4	0.1776	0.0000	OK
15 minute summer	S8	1	224.415	0.000	0.0	0.0000	0.0000	OK
15 minute summer	S9	1	220.750	0.000	0.0	0.0000	0.0000	OK
360 minute summer	S10	240	219.102	0.322	13.2	0.4614	0.0000	SURCHARGED
960 minute summer	S11	810	217.671	0.333	7.7	0.3771	0.0000	SURCHARGED
	S12							
	S13							
360 minute summer	ATTENUATION	240	219.103	0.320	41.3	149.6512	0.0000	OK
960 minute summer	POND	810	217.672	0.329	12.4	178.0933	0.0000	SURCHARGED
960 minute summer	HW1	810	217.363	0.066	7.7	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	S1	1.000	S2	27.1	1.828	0.149	0.3898	
15 minute summer	S2	1.001	S3	27.1	0.900	0.245	0.5153	
15 minute summer	S3	1.002	S4	43.1	1.676	0.170	0.5091	
15 minute summer	S4	1.003	S5	43.0	1.534	0.166	1.1047	
15 minute summer	S5	1.004	S6	84.2	2.703	0.229	2.0600	
15 minute summer	S6	1.005	S7	126.0	3.147	0.220	2.6277	
15 minute summer	S7	1.006	ATTENUATION	177.1	7.138	0.141	0.4788	
15 minute summer	S8	2.000	S9	0.0	0.000	0.000	0.0000	
15 minute summer	S9	2.001	S10	0.0	0.000	0.000	1.4938	
360 minute summer	S10	1.008	POND	12.6	1.691	1.052	0.2511	
960 minute summer	S11	1.010	HW1	7.7	0.635	0.116	0.1362	335.7
360 minute summer	ATTENUATION	1.007	S10	12.0	0.589	0.068	0.1210	
960 minute summer	POND	1.009	S11	7.7	0.130	0.099	0.0704	

Results for 30 year Critical Storm Duration. Lowest mass balance: 99.45%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	S1	9	227.328	0.128	64.1	0.1445	0.0000	OK
15 minute summer	S2	10	226.637	0.212	64.1	0.3033	0.0000	OK
15 minute summer	S3	10	226.553	0.181	101.7	0.2590	0.0000	OK
15 minute summer	S4	10	226.222	0.167	102.0	0.2384	0.0000	OK
15 minute summer	S5	10	225.606	0.206	204.4	0.2942	0.0000	OK
15 minute summer	S6	10	223.352	0.252	304.1	0.3608	0.0000	OK
15 minute summer	S7	10	221.286	0.188	421.5	0.2690	0.0000	OK
15 minute summer	S8	1	224.415	0.000	0.0	0.0000	0.0000	OK
15 minute summer	S9	1	220.750	0.000	0.0	0.0000	0.0000	OK
240 minute winter	S10	228	219.551	0.771	31.2	1.1029	0.0000	SURCHARGED
1440 minute summer	S11	1470	217.883	0.544	7.9	0.6158	0.0000	SURCHARGED
	S12							
	S13							
240 minute winter	ATTENUATION	228	219.551	0.768	77.4	359.5816	0.0000	SURCHARGED
1440 minute summer	POND	1470	217.883	0.540	13.3	315.6876	0.0000	SURCHARGED
600 minute winter	HW1	720	217.364	0.067	7.9	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	S1	1.000	S2	64.1	2.153	0.352	0.7858	
15 minute summer	S2	1.001	S3	64.2	1.099	0.579	0.9970	
15 minute summer	S3	1.002	S4	102.0	2.043	0.402	0.9884	
15 minute summer	S4	1.003	S5	102.4	1.878	0.396	2.1431	
15 minute summer	S5	1.004	S6	202.1	3.374	0.549	3.9587	
15 minute summer	S6	1.005	S7	302.1	3.997	0.528	4.9461	
15 minute summer	S7	1.006	ATTENUATION	423.0	7.590	0.336	1.2971	
15 minute summer	S8	2.000	S9	0.0	0.000	0.000	0.0000	
15 minute summer	S9	2.001	S10	0.0	0.000	0.000	2.5565	
240 minute winter	S10	1.008	POND	14.4	1.846	1.209	0.2511	
1440 minute summer	S11	1.010	HW1	7.9	0.639	0.118	0.1382	557.0
240 minute winter	ATTENUATION	1.007	S10	31.2	-0.559	0.177	0.1584	
1440 minute summer	POND	1.009	S11	7.9	0.130	0.101	0.0704	

Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 99.43%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	S1	9	227.380	0.180	113.6	0.2031	0.0000	OK
15 minute summer	S2	10	226.747	0.322	113.6	0.4601	0.0000	OK
15 minute summer	S3	10	226.639	0.267	180.1	0.3820	0.0000	OK
15 minute summer	S4	10	226.299	0.244	180.1	0.3489	0.0000	OK
15 minute summer	S5	10	225.720	0.320	362.1	0.4583	0.0000	OK
15 minute summer	S6	10	223.506	0.406	542.3	0.5804	0.0000	OK
15 minute summer	S7	10	221.362	0.264	746.8	0.3784	0.0000	OK
15 minute summer	S8	1	224.415	0.000	0.0	0.0000	0.0000	OK
15 minute summer	S9	1	220.750	0.000	0.0	0.0000	0.0000	OK
360 minute winter	S10	344	220.345	1.565	29.0	2.2401	0.0000	SURCHARGED
1440 minute winter	S11	1680	218.161	0.823	7.5	0.9310	0.0000	SURCHARGED
	S12							
	S13							
360 minute winter	ATTENUATION	344	220.347	1.564	104.0	732.3163	0.0000	SURCHARGED
1440 minute winter	POND	1680	218.170	0.827	48.4	532.2880	0.0000	SURCHARGED
360 minute summer	HW1	496	217.364	0.067	7.9	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	S1	1.000	S2	113.6	2.166	0.625	1.3905	
15 minute summer	S2	1.001	S3	113.6	1.225	1.025	1.5745	
15 minute summer	S3	1.002	S4	180.1	2.253	0.710	1.5807	
15 minute summer	S4	1.003	S5	181.2	2.091	0.700	3.4592	
15 minute summer	S5	1.004	S6	361.4	3.610	0.981	6.7136	
15 minute summer	S6	1.005	S7	535.0	4.389	0.935	7.9388	
15 minute summer	S7	1.006	ATTENUATION	747.7	7.713	0.594	1.8931	
15 minute summer	S8	2.000	S9	0.0	0.000	0.000	0.0000	
15 minute summer	S9	2.001	S10	0.0	0.000	0.000	2.5565	
360 minute winter	S10	1.008	POND	17.1	2.189	1.434	0.2511	
1440 minute winter	S11	1.010	HW1	7.9	0.639	0.118	0.1382	608.7
360 minute winter	ATTENUATION	1.007	S10	26.3	-0.563	0.149	0.1584	
1440 minute winter	POND	1.009	S11	-31.8	-0.455	-0.406	0.0704	