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5 Ardent Court William James Way Henley in Arden, B95 5GF	Fole Diary CB COLLIER Network D	
Date 23/07/2025 15:06 File STORM D 02.07.24.MDX	Designed by Ben Lee Checked by	
Innovyze	Network 2020.1	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FEH Rainfall Model	
Return Period (years)	2
FEH Rainfall Version	2013
Site Location	GB 404500 337500 SK 04500 37500
Data Type	Point
Maximum Rainfall (mm/hr)	0
Maximum Time of Concentration (mins)	30
Foul Sewage (l/s/ha)	0.000
Volumetric Runoff Coeff.	0.750
PIMP (%)	100
Add Flow / Climate Change (%)	0
Minimum Backdrop Height (m)	0.000
Maximum Backdrop Height (m)	0.000
Min Design Depth for Optimisation (m)	1.200
Min Vel for Auto Design only (m/s)	1.00
Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.075	4-8	0.048

Total Area Contributing (ha) = 0.123

Total Pipe Volume (m³) = 3.255

Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	9.915	0.950	10.4	0.022	6.00	0.0	0.600	o	150	Pipe/Conduit	
1.001	19.965	0.133	150.1	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.002	11.858	0.079	150.1	0.012	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	0.00	6.05	110.250	0.022	0.0	0.0	0.0	3.14	55.4	0.0
1.001	0.00	6.46	109.300	0.022	0.0	0.0	0.0	0.82	14.5	0.0
1.002	0.00	6.70	109.167	0.034	0.0	0.0	0.0	0.82	14.5	0.0

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Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
2.000	19.149	0.312	61.4	0.040	6.00	0.0	0.600	o	150	Pipe/Conduit	
1.003	5.203	0.035	148.7	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
3.000	12.634	0.547	23.1	0.028	6.00	0.0	0.600	o	150	Pipe/Conduit	
1.004	12.325	0.073	169.0	0.021	0.00	0.0	0.600	o	225	Pipe/Conduit	
4.000	15.000	0.123	122.0	0.000	6.00	0.0	0.600	o	225	Pipe/Conduit	
1.005	11.050	0.065	170.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
1.006	5.600	0.033	169.7	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
2.000	0.00	6.25	109.400	0.040	0.0	0.0	0.0	1.29	22.7	0.0
1.003	0.00	6.78	109.013	0.074	0.0	0.0	0.0	1.07	42.5	0.0
3.000	0.00	6.10	109.600	0.028	0.0	0.0	0.0	2.10	37.2	0.0
1.004	0.00	6.99	108.978	0.123	0.0	0.0	0.0	1.00	39.9	0.0
4.000	0.00	6.21	109.029	0.000	0.0	0.0	0.0	1.18	47.0	0.0
1.005	0.00	7.17	108.905	0.123	0.0	0.0	0.0	1.00	39.8	0.0
1.006	0.00	7.26	108.840	0.123	0.0	0.0	0.0	1.00	39.8	0.0

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
40	110.750	0.500	Open Manhole	475	1.000	110.250	150				
41	110.100	0.800	Open Manhole	475	1.001	109.300	150	1.000	109.300	150	
42	110.650	1.483	Open Manhole	475	1.002	109.167	150	1.001	109.167	150	
43	110.400	1.000	Open Manhole	475	2.000	109.400	150				
44	110.950	1.937	Open Manhole	1200	1.003	109.013	225	1.002	109.088	150	
								2.000	109.088	150	
45	111.400	1.800	Open Manhole	475	3.000	109.600	150				
46	111.150	2.172	Open Manhole	1200	1.004	108.978	225	1.003	108.978	225	
								3.000	109.053	150	
ATT TANK	111.150	2.121	Open Manhole	1200	4.000	109.029	225				
FCC D	111.200	2.295	Open Manhole	1800	1.005	108.905	225	1.004	108.905	225	
								4.000	108.906	225	
47	111.000	2.160	Open Manhole	1200	1.006	108.840	225	1.005	108.840	225	
Outfall	110.600	1.793	Open Manhole	0		OUTFALL		1.006	108.807	225	

No coordinates have been specified, layout information cannot be produced.

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	o	150	40	110.750	110.250	0.350	Open Manhole	475
1.001	o	150	41	110.100	109.300	0.650	Open Manhole	475
1.002	o	150	42	110.650	109.167	1.333	Open Manhole	475
2.000	o	150	43	110.400	109.400	0.850	Open Manhole	475
1.003	o	225	44	110.950	109.013	1.712	Open Manhole	1200
3.000	o	150	45	111.400	109.600	1.650	Open Manhole	475
1.004	o	225	46	111.150	108.978	1.947	Open Manhole	1200
4.000	o	225	ATT TANK	111.150	109.029	1.896	Open Manhole	1200
1.005	o	225	FCC D	111.200	108.905	2.070	Open Manhole	1800
1.006	o	225	47	111.000	108.840	1.935	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	9.915	10.4	41	110.100	109.300	0.650	Open Manhole	475
1.001	19.965	150.1	42	110.650	109.167	1.333	Open Manhole	475
1.002	11.858	150.1	44	110.950	109.088	1.712	Open Manhole	1200
2.000	19.149	61.4	44	110.950	109.088	1.712	Open Manhole	1200
1.003	5.203	148.7	46	111.150	108.978	1.947	Open Manhole	1200
3.000	12.634	23.1	46	111.150	109.053	1.947	Open Manhole	1200
1.004	12.325	169.0	FCC D	111.200	108.905	2.070	Open Manhole	1800
4.000	15.000	122.0	FCC D	111.200	108.906	2.069	Open Manhole	1800
1.005	11.050	170.0	47	111.000	108.840	1.935	Open Manhole	1200
1.006	5.600	169.7	Outfall	110.600	108.807	1.568	Open Manhole	0

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Network Classifications for Storm

PN	USMH Name	Pipe Dia (mm)	Min Cover Depth (m)	Max Cover Depth (m)	Pipe Type	MH Dia (mm)	MH Width (mm)	MH Ring Depth (m)	MH Type
1.000	40	150	0.350	0.650	Unclassified	475	0	0.350	Unclassified
1.001	41	150	0.650	1.333	Unclassified	475	0	0.650	Unclassified
1.002	42	150	1.333	1.712	Unclassified	475	0	1.333	Unclassified
2.000	43	150	0.850	1.712	Unclassified	475	0	0.850	Unclassified
1.003	44	225	1.712	1.947	Unclassified	1200	0	1.712	Unclassified
3.000	45	150	1.650	1.947	Unclassified	475	0	1.650	Unclassified
1.004	46	225	1.947	2.070	Unclassified	1200	0	1.947	Unclassified
4.000	ATT TANK	225	1.896	2.069	Unclassified	1200	0	1.896	Unclassified
1.005	FCC D	225	1.935	2.070	Unclassified	1800	0	2.070	Unclassified
1.006	47	225	1.568	1.935	Unclassified	1200	0	1.935	Unclassified

Surcharged Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
1.006 Outfall		110.600	108.807	0.000	0	0

Datum (m) 0.000 Offset (mins) 0

Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)
1	1.000	24	1.000	47	1.000	70	1.000	93	1.000	116	1.000
2	1.000	25	1.000	48	1.000	71	1.000	94	1.000	117	1.000
3	1.000	26	1.000	49	1.000	72	1.000	95	1.000	118	1.000
4	1.000	27	1.000	50	1.000	73	1.000	96	1.000	119	1.000
5	1.000	28	1.000	51	1.000	74	1.000	97	1.000	120	1.000
6	1.000	29	1.000	52	1.000	75	1.000	98	1.000	121	1.000
7	1.000	30	1.000	53	1.000	76	1.000	99	1.000	122	1.000
8	1.000	31	1.000	54	1.000	77	1.000	100	1.000	123	1.000
9	1.000	32	1.000	55	1.000	78	1.000	101	1.000	124	1.000
10	1.000	33	1.000	56	1.000	79	1.000	102	1.000	125	1.000
11	1.000	34	1.000	57	1.000	80	1.000	103	1.000	126	1.000
12	1.000	35	1.000	58	1.000	81	1.000	104	1.000	127	1.000
13	1.000	36	1.000	59	1.000	82	1.000	105	1.000	128	1.000
14	1.000	37	1.000	60	1.000	83	1.000	106	1.000	129	1.000
15	1.000	38	1.000	61	1.000	84	1.000	107	1.000	130	1.000
16	1.000	39	1.000	62	1.000	85	1.000	108	1.000	131	1.000
17	1.000	40	1.000	63	1.000	86	1.000	109	1.000	132	1.000
18	1.000	41	1.000	64	1.000	87	1.000	110	1.000	133	1.000
19	1.000	42	1.000	65	1.000	88	1.000	111	1.000	134	1.000
20	1.000	43	1.000	66	1.000	89	1.000	112	1.000	135	1.000
21	1.000	44	1.000	67	1.000	90	1.000	113	1.000	136	1.000
22	1.000	45	1.000	68	1.000	91	1.000	114	1.000	137	1.000
23	1.000	46	1.000	69	1.000	92	1.000	115	1.000	138	1.000

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<u>Surcharged Outfall Details for Storm</u>											
Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)
139	1.000	176	1.000	213	1.000	250	1.000	287	1.000	324	1.000
140	1.000	177	1.000	214	1.000	251	1.000	288	1.000	325	1.000
141	1.000	178	1.000	215	1.000	252	1.000	289	1.000	326	1.000
142	1.000	179	1.000	216	1.000	253	1.000	290	1.000	327	1.000
143	1.000	180	1.000	217	1.000	254	1.000	291	1.000	328	1.000
144	1.000	181	1.000	218	1.000	255	1.000	292	1.000	329	1.000
145	1.000	182	1.000	219	1.000	256	1.000	293	1.000	330	1.000
146	1.000	183	1.000	220	1.000	257	1.000	294	1.000	331	1.000
147	1.000	184	1.000	221	1.000	258	1.000	295	1.000	332	1.000
148	1.000	185	1.000	222	1.000	259	1.000	296	1.000	333	1.000
149	1.000	186	1.000	223	1.000	260	1.000	297	1.000	334	1.000
150	1.000	187	1.000	224	1.000	261	1.000	298	1.000	335	1.000
151	1.000	188	1.000	225	1.000	262	1.000	299	1.000	336	1.000
152	1.000	189	1.000	226	1.000	263	1.000	300	1.000	337	1.000
153	1.000	190	1.000	227	1.000	264	1.000	301	1.000	338	1.000
154	1.000	191	1.000	228	1.000	265	1.000	302	1.000	339	1.000
155	1.000	192	1.000	229	1.000	266	1.000	303	1.000	340	1.000
156	1.000	193	1.000	230	1.000	267	1.000	304	1.000	341	1.000
157	1.000	194	1.000	231	1.000	268	1.000	305	1.000	342	1.000
158	1.000	195	1.000	232	1.000	269	1.000	306	1.000	343	1.000
159	1.000	196	1.000	233	1.000	270	1.000	307	1.000	344	1.000
160	1.000	197	1.000	234	1.000	271	1.000	308	1.000	345	1.000
161	1.000	198	1.000	235	1.000	272	1.000	309	1.000	346	1.000
162	1.000	199	1.000	236	1.000	273	1.000	310	1.000	347	1.000
163	1.000	200	1.000	237	1.000	274	1.000	311	1.000	348	1.000
164	1.000	201	1.000	238	1.000	275	1.000	312	1.000	349	1.000
165	1.000	202	1.000	239	1.000	276	1.000	313	1.000	350	1.000
166	1.000	203	1.000	240	1.000	277	1.000	314	1.000	351	1.000
167	1.000	204	1.000	241	1.000	278	1.000	315	1.000	352	1.000
168	1.000	205	1.000	242	1.000	279	1.000	316	1.000	353	1.000
169	1.000	206	1.000	243	1.000	280	1.000	317	1.000	354	1.000
170	1.000	207	1.000	244	1.000	281	1.000	318	1.000	355	1.000
171	1.000	208	1.000	245	1.000	282	1.000	319	1.000	356	1.000
172	1.000	209	1.000	246	1.000	283	1.000	320	1.000	357	1.000
173	1.000	210	1.000	247	1.000	284	1.000	321	1.000	358	1.000
174	1.000	211	1.000	248	1.000	285	1.000	322	1.000	359	1.000
175	1.000	212	1.000	249	1.000	286	1.000	323	1.000	360	1.000
<u>Simulation Criteria for Storm</u>											
Volumetric Runoff Coeff 0.750				Additional Flow - % of Total Flow 0.000							
Areal Reduction Factor 1.000				MADD Factor * 10m³/ha Storage 2.000							
Hot Start (mins) 0				Inlet Coeffiecient 0.800							
Hot Start Level (mm) 0				Flow per Person per Day (l/per/day) 0.000							
Manhole Headloss Coeff (Global) 0.500				Run Time (mins) 60							
Foul Sewage per hectare (l/s) 0.000				Output Interval (mins) 1							
Number of Input Hydrographs 0				Number of Offline Controls 0							
Number of Online Controls 1				Number of Storage Structures 2							
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Simulation Criteria for Storm Number of Time/Area Diagrams 0 Number of Real Time Controls 0 Synthetic Rainfall Details <table><tr><td>Rainfall Model</td><td>FSR</td><td>Profile Type</td><td>Summer</td></tr><tr><td>Return Period (years)</td><td>100</td><td>Cv (Summer)</td><td>0.750</td></tr><tr><td>Region</td><td>England and Wales</td><td>Cv (Winter)</td><td>0.840</td></tr><tr><td>M5-60 (mm)</td><td>19.000</td><td>Storm Duration (mins)</td><td>30</td></tr><tr><td>Ratio R</td><td>0.373</td><td></td><td></td></tr></table>			Rainfall Model	FSR	Profile Type	Summer	Return Period (years)	100	Cv (Summer)	0.750	Region	England and Wales	Cv (Winter)	0.840	M5-60 (mm)	19.000	Storm Duration (mins)	30	Ratio R	0.373		
Rainfall Model	FSR	Profile Type	Summer																			
Return Period (years)	100	Cv (Summer)	0.750																			
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Online Controls for Storm

Hydro-Brake® Optimum Manhole: FCC D, DS/PN: 1.005, Volume (m³): 6.8

Unit Reference	MD-SHE-0199-2000-1000-2000
Design Head (m)	1.000
Design Flow (l/s)	20.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	199
Invert Level (m)	108.905
Minimum Outlet Pipe Diameter (mm)	225
Suggested Manhole Diameter (mm)	1500

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.000	20.0
Flush-Flo™	0.340	20.0
Kick-Flo®	0.721	17.1
Mean Flow over Head Range	-	16.8

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	6.9	1.200	21.8	3.000	33.8	7.000	51.0
0.200	18.7	1.400	23.5	3.500	36.4	7.500	52.7
0.300	19.9	1.600	25.0	4.000	38.9	8.000	54.4
0.400	19.9	1.800	26.5	4.500	41.1	8.500	56.0
0.500	19.5	2.000	27.8	5.000	43.3	9.000	57.6
0.600	19.0	2.200	29.1	5.500	45.3	9.500	59.1
0.800	18.0	2.400	30.4	6.000	47.3		
1.000	20.0	2.600	31.6	6.500	49.2		

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<u>Storage Structures for Storm</u> <u>Porous Car Park Manhole: 42, DS/PN: 1.002</u> <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>6.0</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>71.2</td></tr><tr><td>Max Percolation (l/s)</td><td>118.7</td><td>Slope (1:X)</td><td>36.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>0</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>0</td></tr><tr><td>Invert Level (m)</td><td>109.090</td><td>Cap Volume Depth (m)</td><td>0.350</td></tr></table> <u>Tank or Pond Manhole: ATT TANK, DS/PN: 4.000</u> Invert Level (m) 109.029 <table><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr><tr><td>0.000</td><td>60.0</td><td>0.700</td><td>0.0</td><td>1.400</td><td>0.0</td><td>2.100</td><td>0.0</td></tr><tr><td>0.100</td><td>60.0</td><td>0.800</td><td>0.0</td><td>1.500</td><td>0.0</td><td>2.200</td><td>0.0</td></tr><tr><td>0.200</td><td>60.0</td><td>0.900</td><td>0.0</td><td>1.600</td><td>0.0</td><td>2.300</td><td>0.0</td></tr><tr><td>0.300</td><td>60.0</td><td>1.000</td><td>0.0</td><td>1.700</td><td>0.0</td><td>2.400</td><td>0.0</td></tr><tr><td>0.400</td><td>60.0</td><td>1.100</td><td>0.0</td><td>1.800</td><td>0.0</td><td>2.500</td><td>0.0</td></tr><tr><td>0.401</td><td>0.0</td><td>1.200</td><td>0.0</td><td>1.900</td><td>0.0</td><td></td><td></td></tr><tr><td>0.600</td><td>0.0</td><td>1.300</td><td>0.0</td><td>2.000</td><td>0.0</td><td></td><td></td></tr></table>			Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	6.0	Membrane Percolation (mm/hr)	1000	Length (m)	71.2	Max Percolation (l/s)	118.7	Slope (1:X)	36.0	Safety Factor	2.0	Depression Storage (mm)	0	Porosity	0.30	Evaporation (mm/day)	0	Invert Level (m)	109.090	Cap Volume Depth (m)	0.350	Depth (m)	Area (m²)	Depth (m)	Area (m²)	Depth (m)	Area (m²)	Depth (m)	Area (m²)	0.000	60.0	0.700	0.0	1.400	0.0	2.100	0.0	0.100	60.0	0.800	0.0	1.500	0.0	2.200	0.0	0.200	60.0	0.900	0.0	1.600	0.0	2.300	0.0	0.300	60.0	1.000	0.0	1.700	0.0	2.400	0.0	0.400	60.0	1.100	0.0	1.800	0.0	2.500	0.0	0.401	0.0	1.200	0.0	1.900	0.0			0.600	0.0	1.300	0.0	2.000	0.0		
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Henley in Arden, B95 5GF

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Fole Diary
CB COLLIER
Network D

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1000 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor 1.000

Additional Flow - % of Total Flow 0.000

Hot Start (mins) 0

MADD Factor * 10m³/ha Storage 2.000

Hot Start Level (mm) 0

Inlet Coefficient 0.800

Manhole Headloss Coeff (Global) 0.500

Flow per Person per Day (l/per/day) 0.000

Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0

Number of Storage Structures 2

Number of Online Controls 1

Number of Time/Area Diagrams 0

Number of Offline Controls 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR

Ratio R 0.373

Region England and Wales Cv (Summer) 0.750

M5-60 (mm) 19.000

Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status ON

DVD Status ON

Inertia Status ON

Profile(s) Summer and Winter

Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440

Return Period(s) (years) 1000

Climate Change (%) 0

PN	US/MH Name	Storm	Return Climate Period Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.
1.000	40 15 Winter	1000	+0%				
1.001	41 15 Winter	1000	+0%	1000/15 Summer			
1.002	42 15 Winter	1000	+0%	1000/15 Summer			
2.000	43 15 Winter	1000	+0%	1000/15 Summer			
1.003	44 15 Winter	1000	+0%	1000/15 Summer			
3.000	45 15 Winter	1000	+0%				
1.004	46 15 Winter	1000	+0%	1000/15 Summer			
4.000	ATT TANK 30 Winter	1000	+0%	1000/30 Summer			
1.005	FCC D 15 Winter	1000	+0%	1000/15 Summer			
1.006	47 15 Summer	1000	+0%				

PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap. (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status
1.000	40 110.303	-0.097	0.000	0.28			13.6	OK
1.001	41 109.689	0.239	0.000	0.97			13.2	SURCHARGED
1.002	42 109.601	0.284	0.000	1.35		8	17.7	SURCHARGED

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1000 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status
2.000	43	109.943	0.393	0.000	1.11			23.6	SURCHARGED
1.003	44	109.558	0.320	0.000	1.00			30.0	SURCHARGED
3.000	45	109.676	-0.074	0.000	0.51			17.3	OK
1.004	46	109.491	0.288	0.000	1.44			49.2	SURCHARGED
4.000	ATT TANK	109.301	0.047	0.000	0.34			14.1	SURCHARGED
1.005	FCC D	109.332	0.202	0.000	0.59			19.9	SURCHARGED
1.006	47	108.975	-0.090	0.000	0.67			19.8	OK

PN	US/MH Name	Level Exceeded
1.000	40	
1.001	41	
1.002	42	
2.000	43	
1.003	44	
3.000	45	
1.004	46	
4.000	ATT TANK	
1.005	FCC D	
1.006	47	