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Ebenezer House Ryecroft Newcastle ST5 2BE	Royal Oak Drainage Design 1 in 100 30CC	
Date 13/12/2024 12:59 File 2024.12.13 Network Rev ...	Designed by F. Gennard Checked by	
Innovyze	Network 2020.1.3	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	18.000	Add Flow / Climate Change (%)	0
Ratio R	0.357	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	22.190	0.285	77.9	0.034	5.00	0.0	0.600	o	100	Pipe/Conduit	
1.001	11.998	0.575	20.9	0.000	0.00	0.0	0.600	o	100	Pipe/Conduit	
2.000	22.849	0.238	96.0	0.027	5.00	0.0	0.600	o	100	Pipe/Conduit	
3.000	12.000	0.200	60.0	0.021	5.00	0.0	0.600	o	100	Pipe/Conduit	
3.001	10.535	0.105	100.3	0.008	0.00	0.0	0.600	o	100	Pipe/Conduit	
1.002	7.028	0.154	45.6	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
4.000	45.018	0.113	400.0	0.059	5.00	0.0	0.600	o	150	Pipe/Conduit	
4.001	2.873	0.029	99.1	0.006	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.00	5.42	214.274	0.034	0.0	0.0	0.0	0.87	6.9	4.7
1.001	50.00	5.54	213.989	0.034	0.0	0.0	0.0	1.70	13.3	4.7
2.000	50.00	5.49	213.652	0.027	0.0	0.0	0.0	0.78	6.2	3.7
3.000	50.00	5.20	213.720	0.021	0.0	0.0	0.0	1.00	7.8	2.8
3.001	50.00	5.43	213.520	0.029	0.0	0.0	0.0	0.77	6.0	3.9
1.002	50.00	5.62	213.365	0.091	0.0	0.0	0.0	1.49	26.4	12.3
4.000	50.00	6.51	213.353	0.059	0.0	0.0	0.0	0.50	8.8	8.0
4.001	50.00	6.56	213.240	0.065	0.0	0.0	0.0	1.01	17.8	8.7

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Area Summary for Storm

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.000	User	-	100	0.034	0.034	0.034
1.001	-	-	100	0.000	0.000	0.000
2.000	User	-	100	0.027	0.027	0.027
3.000	User	-	100	0.021	0.021	0.021
3.001	User	-	100	0.008	0.008	0.008
1.002	-	-	100	0.000	0.000	0.000
4.000	User	-	100	0.021	0.021	0.021
	User	-	100	0.007	0.007	0.028
	User	-	100	0.007	0.007	0.035
	User	-	100	0.007	0.007	0.041
	User	-	100	0.007	0.007	0.048
	User	-	100	0.011	0.011	0.059
4.001	User	-	100	0.006	0.006	0.006
1.003	-	-	100	0.000	0.000	0.000
				Total	Total	Total
				0.155	0.155	0.155

Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
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1.003		213.960	213.170	213.160	0	0
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Simulation Criteria for Storm

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m ³ /ha Storage	2.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs	0	Number of Storage Structures	4
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	18.000	Storm Duration (mins)	30
Ratio R	0.357		

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Online Controls for Storm

Hydro-Brake® Optimum Manhole: 9, DS/PN: 1.003, Volume (m³): 1.0

Unit Reference	MD-SHE-0109-5000-0800-5000
Design Head (m)	0.800
Design Flow (l/s)	5.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	109
Invert Level (m)	213.211
Minimum Outlet Pipe Diameter (mm)	150
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.800	5.0
Flush-Flo™	0.241	5.0
Kick-Flo®	0.537	4.2
Mean Flow over Head Range	-	4.3

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	3.7	1.200	6.0	3.000	9.3	7.000	13.9
0.200	5.0	1.400	6.5	3.500	10.0	7.500	14.4
0.300	5.0	1.600	6.9	4.000	10.6	8.000	14.8
0.400	4.8	1.800	7.3	4.500	11.3	8.500	15.2
0.500	4.5	2.000	7.7	5.000	11.8	9.000	15.7
0.600	4.4	2.200	8.0	5.500	12.4	9.500	16.1
0.800	5.0	2.400	8.4	6.000	12.9		
1.000	5.5	2.600	8.7	6.500	13.4		

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Storage Structures for Storm

Porous Car Park Manhole: 1, DS/PN: 1.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	5.0
Membrane Percolation (mm/hr)	1000	Length (m)	22.0
Max Percolation (l/s)	30.6	Slope (1:X)	78.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	214.274	Membrane Depth (mm)	0

Porous Car Park Manhole: 3, DS/PN: 2.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	8.0
Membrane Percolation (mm/hr)	1000	Length (m)	23.0
Max Percolation (l/s)	51.1	Slope (1:X)	96.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	213.652	Membrane Depth (mm)	0

Porous Car Park Manhole: 4, DS/PN: 3.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	4.8
Membrane Percolation (mm/hr)	1000	Length (m)	12.0
Max Percolation (l/s)	16.0	Slope (1:X)	60.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	213.720	Membrane Depth (mm)	0

Porous Car Park Manhole: 7, DS/PN: 4.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	4.2
Membrane Percolation (mm/hr)	1000	Length (m)	46.0
Max Percolation (l/s)	53.7	Slope (1:X)	400.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	213.353	Membrane Depth (mm)	0

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Ryecroft

Newcastle ST5 2BE

Royal Oak

Drainage Design

1 in 100 30CC

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Network 2020.1.3

1 year Return Period Summary of Critical Results by Maximum Flood Volume

(Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor 1.000

Additional Flow - % of Total Flow 0.000

Hot Start (mins) 0

MADD Factor * 10m³/ha Storage 2.000

Hot Start Level (mm) 0

Inlet Coefficient 0.800

Manhole Headloss Coeff (Global) 0.500

Flow per Person per Day (l/per/day) 0.000

Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0

Number of Storage Structures 4

Number of Online Controls 1

Number of Time/Area Diagrams 0

Number of Offline Controls 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR

Ratio R 0.356

Region England and Wales Cv (Summer) 0.750

M5-60 (mm) 18.000

Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status ON

DVD Status OFF

Inertia Status OFF

Profile(s) Summer and Winter

Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440

Return Period(s) (years) 1, 30, 100

Climate Change (%) 0, 0, 30

US/MH

PN

Name

Storm

Return Period

Climate Change

First (X) Surcharge

First (Y) Flood

First (Z) Overflow

Overflow Act.

Water Level (m)

1.000

1 15 Summer

1

+0%

30/15 Summer

214.326

1.001

2 15 Summer

1

+0%

100/30 Winter

214.024

2.000

3 15 Summer

1

+0%

30/15 Summer

213.684

3.000

4 15 Summer

1

+0%

30/15 Summer

213.758

3.001

5 15 Summer

1

+0%

30/15 Summer

213.573

1.002

6 15 Summer

1

+0%

1/30 Winter

213.450

4.000

7 15 Summer

1

+0%

30/15 Summer

213.426

4.001

8 15 Summer

1

+0%

1/15 Summer

213.437

1.003

9 15 Summer

1

+0%

1/15 Summer

213.437

Surcharged Flooded

Half Drain Pipe

US/MH

PN

Name

Depth (m)

Volume (m³)

Flow / Cap.

Overflow (l/s)

Time (mins)

Pipe Flow (l/s)

Status

Level Exceeded

1.000

1

-0.048

0.000

0.52

4

3.5

OK

1.001

2

-0.065

0.000

0.27

3.4

OK

2.000

3

-0.068

0.000

0.22

5

1.3

OK

3.000

4

-0.062

0.000

0.31

5

2.3

OK

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1 year Return Period Summary of Critical Results by Maximum Flood Volume
(Rank 1) for Storm

PN	US/MH Name	Surcharged		Flooded		Half Drain		Pipe	Status	Level Exceeded
		Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Time (mins)	Flow (l/s)	Flow		
3.001	5	-0.047	0.000	0.55			3.1		OK	
1.002	6	-0.065	0.000	0.30			6.9		OK	
4.000	7	-0.077	0.000	0.31		7	2.7		OK	
4.001	8	0.047	0.000	0.35			3.8	SURCHARGED		
1.003	9	0.076	0.000	0.39			5.0	SURCHARGED		

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30 year Return Period Summary of Critical Results by Maximum Flood Volume
(Rank 1) for Storm

PN	US/MH Name	Surcharged		Flooded		Half Drain		Pipe	Status	Level Exceeded
		Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Time (mins)	Flow (l/s)	Flow		
3.001	5	0.188	0.000	1.14			6.4	FLOOD RISK		
1.002	6	0.261	0.000	0.58			13.1	SURCHARGED		
4.000	7	0.070	0.000	0.56		22	4.7	SURCHARGED		
4.001	8	0.327	0.000	0.45			4.9	FLOOD RISK		
1.003	9	0.389	0.000	0.39			5.0	FLOOD RISK		

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Ryecroft

Newcastle ST5 2BE

Royal Oak

Drainage Design

1 in 100 30CC

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100 year Return Period Summary of Critical Results by Maximum Flood Volume

(Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor 1.000

Additional Flow - % of Total Flow 0.000

Hot Start (mins) 0

MADD Factor * 10m³/ha Storage 2.000

Hot Start Level (mm) 0

Inlet Coefficient 0.800

Manhole Headloss Coeff (Global) 0.500

Flow per Person per Day (l/per/day) 0.000

Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0

Number of Storage Structures 4

Number of Online Controls 1

Number of Time/Area Diagrams 0

Number of Offline Controls 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR

Ratio R 0.356

Region England and Wales Cv (Summer) 0.750

M5-60 (mm) 18.000

Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status ON

DVD Status OFF

Inertia Status OFF

Profile(s) Summer and Winter

Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440

Return Period(s) (years) 1, 30, 100

Climate Change (%) 0, 0, 30

WARNING: Half Drain Time has not been calculated as the structure is too full.

US/MH

PN

Name

Storm

Return

Period

Climate

Change

First (X)

Surcharge

First (Y)

Flood

First (Z)

Overflow

Overflow

Act.

Water

Level

1.000

1

15 Summer

100

+30%

30/15 Summer

214.473

1.001

2

15 Summer

100

+30%

100/30 Winter

214.047

2.000

3

15 Summer

100

+30%

30/15 Summer

213.841

3.000

4

15 Summer

100

+30%

30/15 Summer

213.932

3.001

5

15 Summer

100

+30%

30/15 Summer

213.896

1.002

6

15 Summer

100

+30%

1/30 Winter

213.837

4.000

7

15 Summer

100

+30%

30/15 Summer

213.704

4.001

8

15 Summer

100

+30%

1/15 Summer

213.772

1.003

9

15 Summer

100

+30%

1/15 Summer

213.803

Surcharged

Flooded

US/MH

PN

Depth

(m)

Volume

(m³)

Flow /

Cap.

Overflow

(l/s)

Half Drain

Time

Pipe

Flow

Status

Exceeded

1.000

1

0.099

0.000

1.17

6

7.7

SURCHARGED

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100 year Return Period Summary of Critical Results by Maximum Flood Volume
(Rank 1) for Storm

PN	US/MH Name	Surcharged		Flooded		Half Drain		Pipe	Status	Level Exceeded
		Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Time (mins)	Flow (l/s)	Flow		
1.001	2	-0.042	0.000	0.62			7.7		OK	
2.000	3	0.089	0.000	0.61		15	3.6		SURCHARGED	
3.000	4	0.112	0.000	0.67		10	4.9		SURCHARGED	
3.001	5	0.276	0.000	1.17			6.6		FLOOD RISK	
1.002	6	0.322	0.000	0.59			13.3		SURCHARGED	
4.000	7	0.201	0.000	0.54			4.6		FLOOD RISK	
4.001	8	0.382	0.000	0.43			4.6		FLOOD RISK	
1.003	9	0.442	0.000	0.39			4.9		FLOOD RISK	

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Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

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Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	18.000	Add Flow / Climate Change (%)	0
Ratio R	0.357	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	22.190	0.285	77.9	0.034	5.00	0.0	0.600	o	100	Pipe/Conduit	
1.001	11.998	0.575	20.9	0.000	0.00	0.0	0.600	o	100	Pipe/Conduit	
2.000	22.849	0.238	96.0	0.027	5.00	0.0	0.600	o	100	Pipe/Conduit	
3.000	12.000	0.200	60.0	0.021	5.00	0.0	0.600	o	100	Pipe/Conduit	
3.001	10.535	0.105	100.3	0.008	0.00	0.0	0.600	o	100	Pipe/Conduit	
1.002	7.028	0.154	45.6	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
4.000	45.018	0.113	400.0	0.059	5.00	0.0	0.600	o	150	Pipe/Conduit	
4.001	2.873	0.029	99.1	0.006	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.00	5.42	214.274	0.034	0.0	0.0	0.0	0.87	6.9	4.7
1.001	50.00	5.54	213.989	0.034	0.0	0.0	0.0	1.70	13.3	4.7
2.000	50.00	5.49	213.652	0.027	0.0	0.0	0.0	0.78	6.2	3.7
3.000	50.00	5.20	213.720	0.021	0.0	0.0	0.0	1.00	7.8	2.8
3.001	50.00	5.43	213.520	0.029	0.0	0.0	0.0	0.77	6.0	3.9
1.002	50.00	5.62	213.365	0.091	0.0	0.0	0.0	1.49	26.4	12.3
4.000	50.00	6.51	213.353	0.059	0.0	0.0	0.0	0.50	8.8	8.0
4.001	50.00	6.56	213.240	0.065	0.0	0.0	0.0	1.01	17.8	8.7

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Area Summary for Storm

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.000	User	-	100	0.034	0.034	0.034
1.001	-	-	100	0.000	0.000	0.000
2.000	User	-	100	0.027	0.027	0.027
3.000	User	-	100	0.021	0.021	0.021
3.001	User	-	100	0.008	0.008	0.008
1.002	-	-	100	0.000	0.000	0.000
4.000	User	-	100	0.021	0.021	0.021
	User	-	100	0.007	0.007	0.028
	User	-	100	0.007	0.007	0.035
	User	-	100	0.007	0.007	0.041
	User	-	100	0.007	0.007	0.048
	User	-	100	0.011	0.011	0.059
4.001	User	-	100	0.006	0.006	0.006
1.003	-	-	100	0.000	0.000	0.000
				Total	Total	Total
				0.155	0.155	0.155

Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
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1.003		213.960	213.170	213.160	0	0
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Simulation Criteria for Storm

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m ³ /ha Storage	2.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs	0	Number of Storage Structures	4
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	18.000	Storm Duration (mins)	30
Ratio R	0.357		

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Ebenezer House Ryecroft Newcastle ST5 2BE	Royal Oak Drainage Design 1 in 100 40CC																																																																																																														
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<u>Online Controls for Storm</u> <u>Hydro-Brake® Optimum Manhole: 9, DS/PN: 1.003, Volume (m³): 1.0</u> <table><tr><td>Unit Reference</td><td>MD-SHE-0109-5000-0800-5000</td></tr><tr><td>Design Head (m)</td><td>0.800</td></tr><tr><td>Design Flow (l/s)</td><td>5.0</td></tr><tr><td>Flush-Flo™</td><td>Calculated</td></tr><tr><td>Objective</td><td>Minimise upstream storage</td></tr><tr><td>Application</td><td>Surface</td></tr><tr><td>Sump Available</td><td>Yes</td></tr><tr><td>Diameter (mm)</td><td>109</td></tr><tr><td>Invert Level (m)</td><td>213.211</td></tr><tr><td>Minimum Outlet Pipe Diameter (mm)</td><td>150</td></tr><tr><td>Suggested Manhole Diameter (mm)</td><td>1200</td></tr></table> <table><tr><td>Control Points</td><td>Head (m)</td><td>Flow (l/s)</td></tr><tr><td>Design Point (Calculated)</td><td>0.800</td><td>5.0</td></tr><tr><td>Flush-Flo™</td><td>0.241</td><td>5.0</td></tr><tr><td>Kick-Flo®</td><td>0.537</td><td>4.2</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>4.3</td></tr></table> The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated <table><tr><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td></tr><tr><td>0.100</td><td>3.7</td><td>1.200</td><td>6.0</td><td>3.000</td><td>9.3</td><td>7.000</td><td>13.9</td></tr><tr><td>0.200</td><td>5.0</td><td>1.400</td><td>6.5</td><td>3.500</td><td>10.0</td><td>7.500</td><td>14.4</td></tr><tr><td>0.300</td><td>5.0</td><td>1.600</td><td>6.9</td><td>4.000</td><td>10.6</td><td>8.000</td><td>14.8</td></tr><tr><td>0.400</td><td>4.8</td><td>1.800</td><td>7.3</td><td>4.500</td><td>11.3</td><td>8.500</td><td>15.2</td></tr><tr><td>0.500</td><td>4.5</td><td>2.000</td><td>7.7</td><td>5.000</td><td>11.8</td><td>9.000</td><td>15.7</td></tr><tr><td>0.600</td><td>4.4</td><td>2.200</td><td>8.0</td><td>5.500</td><td>12.4</td><td>9.500</td><td>16.1</td></tr><tr><td>0.800</td><td>5.0</td><td>2.400</td><td>8.4</td><td>6.000</td><td>12.9</td><td></td><td></td></tr><tr><td>1.000</td><td>5.5</td><td>2.600</td><td>8.7</td><td>6.500</td><td>13.4</td><td></td><td></td></tr></table>			Unit Reference	MD-SHE-0109-5000-0800-5000	Design Head (m)	0.800	Design Flow (l/s)	5.0	Flush-Flo™	Calculated	Objective	Minimise upstream storage	Application	Surface	Sump Available	Yes	Diameter (mm)	109	Invert Level (m)	213.211	Minimum Outlet Pipe Diameter (mm)	150	Suggested Manhole Diameter (mm)	1200	Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	0.800	5.0	Flush-Flo™	0.241	5.0	Kick-Flo®	0.537	4.2	Mean Flow over Head Range	-	4.3	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	3.7	1.200	6.0	3.000	9.3	7.000	13.9	0.200	5.0	1.400	6.5	3.500	10.0	7.500	14.4	0.300	5.0	1.600	6.9	4.000	10.6	8.000	14.8	0.400	4.8	1.800	7.3	4.500	11.3	8.500	15.2	0.500	4.5	2.000	7.7	5.000	11.8	9.000	15.7	0.600	4.4	2.200	8.0	5.500	12.4	9.500	16.1	0.800	5.0	2.400	8.4	6.000	12.9			1.000	5.5	2.600	8.7	6.500	13.4		
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Storage Structures for Storm

Porous Car Park Manhole: 1, DS/PN: 1.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	5.0
Membrane Percolation (mm/hr)	1000	Length (m)	22.0
Max Percolation (l/s)	30.6	Slope (1:X)	78.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	214.274	Membrane Depth (mm)	0

Porous Car Park Manhole: 3, DS/PN: 2.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	8.0
Membrane Percolation (mm/hr)	1000	Length (m)	23.0
Max Percolation (l/s)	51.1	Slope (1:X)	96.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	213.652	Membrane Depth (mm)	0

Porous Car Park Manhole: 4, DS/PN: 3.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	4.8
Membrane Percolation (mm/hr)	1000	Length (m)	12.0
Max Percolation (l/s)	16.0	Slope (1:X)	60.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	213.720	Membrane Depth (mm)	0

Porous Car Park Manhole: 7, DS/PN: 4.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	4.2
Membrane Percolation (mm/hr)	1000	Length (m)	46.0
Max Percolation (l/s)	53.7	Slope (1:X)	400.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	213.353	Membrane Depth (mm)	0

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1 year Return Period Summary of Critical Results by Maximum Flood Volume
(Rank 1) for Storm

PN	US/MH Name	Surcharged		Flooded		Half Drain		Pipe	Status	Level Exceeded
		Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Time (mins)	Flow (l/s)	Flow		
3.001	5	-0.047	0.000	0.55			3.1		OK	
1.002	6	-0.065	0.000	0.30			6.9		OK	
4.000	7	-0.077	0.000	0.31		7	2.7		OK	
4.001	8	0.047	0.000	0.35			3.8	SURCHARGED		
1.003	9	0.076	0.000	0.39			5.0	SURCHARGED		

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Ryecroft

Newcastle ST5 2BE

Royal Oak

Drainage Design

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30 year Return Period Summary of Critical Results by Maximum Flood Volume

(Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor 1.000

Additional Flow - % of Total Flow 0.000

Hot Start (mins) 0

MADD Factor * 10m³/ha Storage 2.000

Hot Start Level (mm) 0

Inlet Coefficient 0.800

Manhole Headloss Coeff (Global) 0.500

Flow per Person per Day (l/per/day) 0.000

Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0

Number of Storage Structures 4

Number of Online Controls 1

Number of Time/Area Diagrams 0

Number of Offline Controls 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR

Ratio R 0.356

Region England and Wales Cv (Summer) 0.750

M5-60 (mm) 18.000 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status ON

DVD Status OFF

Inertia Status OFF

Profile(s) Summer and Winter

Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440

Return Period(s) (years) 1, 30, 100

Climate Change (%) 0, 0, 40

US/MH

PN

Name

Storm

Return Period

Climate Change

First (X) Surcharge

First (Y) Flood

First (Z) Overflow

Overflow Act.

Water Level (m)

1.000

1

15 Summer

30

+0%

30/15 Summer

214.388

1.001

2

15 Summer

30

+0%

100/15 Winter

214.042

2.000

3

15 Summer

30

+0%

30/15 Summer

213.763

3.000

4

15 Summer

30

+0%

30/15 Summer

213.831

3.001

5

15 Summer

30

+0%

30/15 Summer

213.808

1.002

6

15 Summer

30

+0%

1/30 Winter

213.776

4.000

7

15 Summer

30

+0%

30/15 Summer

213.573

4.001

8

15 Summer

30

+0%

1/15 Summer

213.717

1.003

9

15 Summer

30

+0%

1/15 Summer

213.750

Surcharged Flooded

Half Drain

Pipe

US/MH

PN

Name

Depth (m)

Volume (m³)

Flow / Cap.

Overflow (l/s)

Time (mins)

Flow (l/s)

Status

Level Exceeded

1.000

1

0.014

0.000

1.03

4

6.8 SURCHARGED

1.001

2

-0.047

0.000

0.54

6.8 OK

2.000

3

0.011

0.000

0.73

6

4.4 SURCHARGED

3.000

4

0.011

0.000

0.68

5

5.0 SURCHARGED

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30 year Return Period Summary of Critical Results by Maximum Flood Volume
(Rank 1) for Storm

PN	US/MH Name	Surcharged		Flooded		Flow / Overflow Cap. (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
		Depth (m)	Volume (m³)	Flow	Overflow					
3.001	5	0.188	0.000	1.14				6.4	FLOOD RISK	
1.002	6	0.261	0.000	0.58				13.1	SURCHARGED	
4.000	7	0.070	0.000	0.56			22	4.7	SURCHARGED	
4.001	8	0.327	0.000	0.45				4.9	FLOOD RISK	
1.003	9	0.389	0.000	0.39				5.0	FLOOD RISK	